

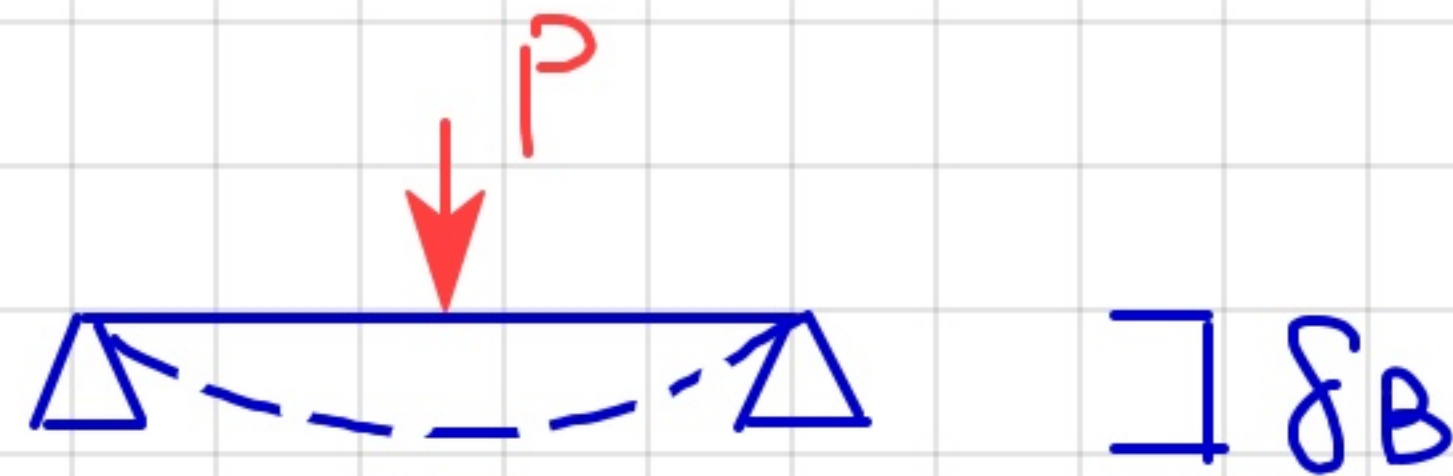
No.1 ため問題 (H30年の類似)

$\delta_A : \delta_B$ を求める

公式に代入して求める



$$\delta_A = \frac{PL^3}{3EI}$$



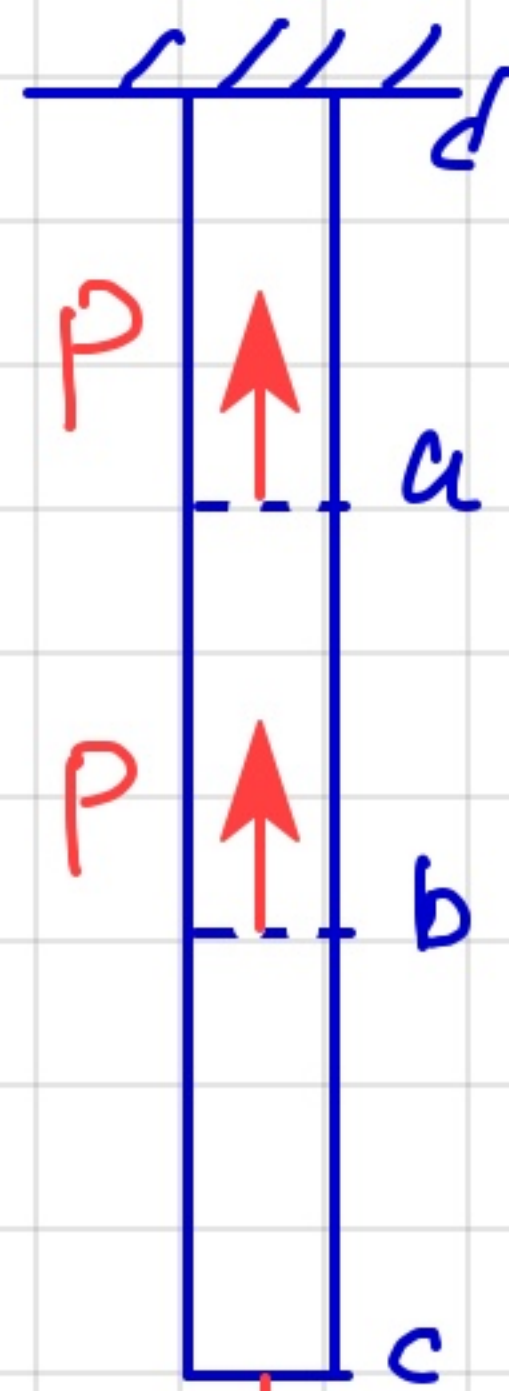
$$\delta_B = \frac{PL^3}{48EI}$$



$$\begin{aligned} \delta_A : \delta_B &= \frac{PL^3}{3EI} : \frac{PL^3}{48EI} \quad \times 16 \quad \times 16 \\ &= \frac{1}{3} : \frac{1}{48} \\ &= 16 : 1 \end{aligned}$$

No2, 変位の問題

a-b 間の軸力, cの軸方向変位

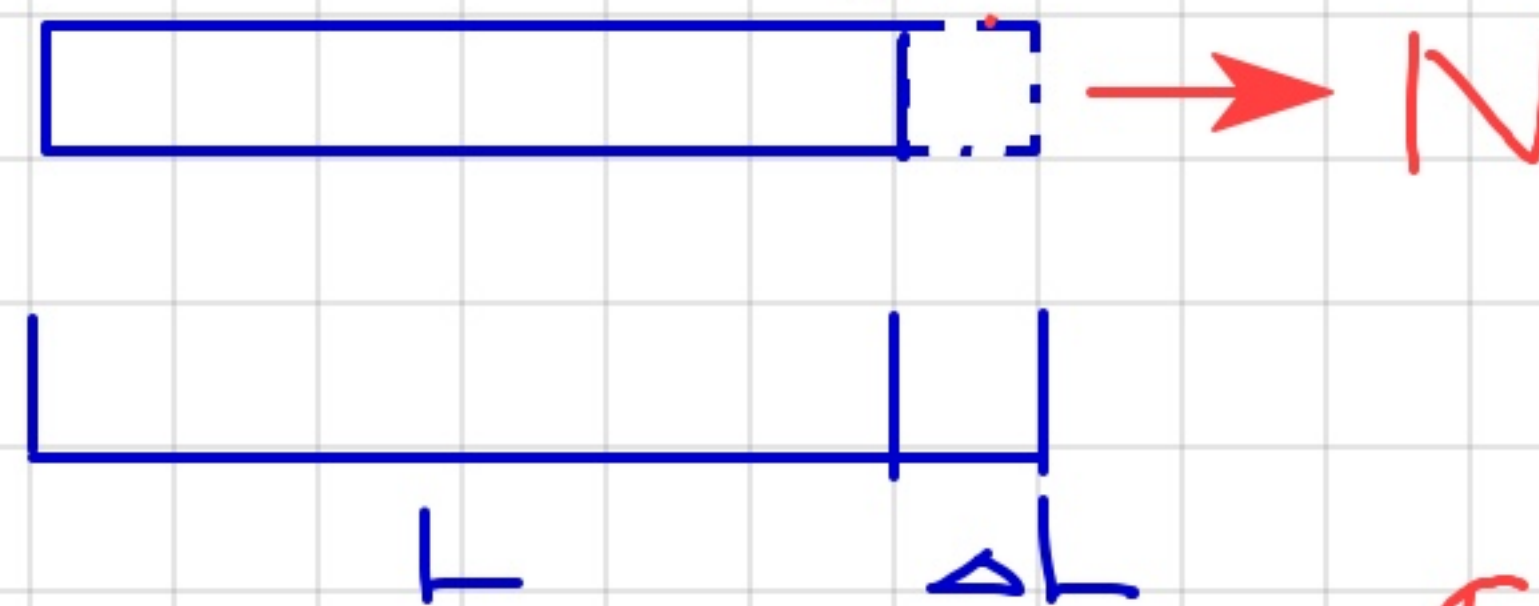


$$\begin{aligned}\Delta ad &= 0 \\ \Delta ab &= \frac{PL}{AE} \\ \Delta bc &= \frac{2PL}{AE}\end{aligned}$$

$$\text{Total displacement at c} = \frac{3PL}{AE}$$

変位の公式

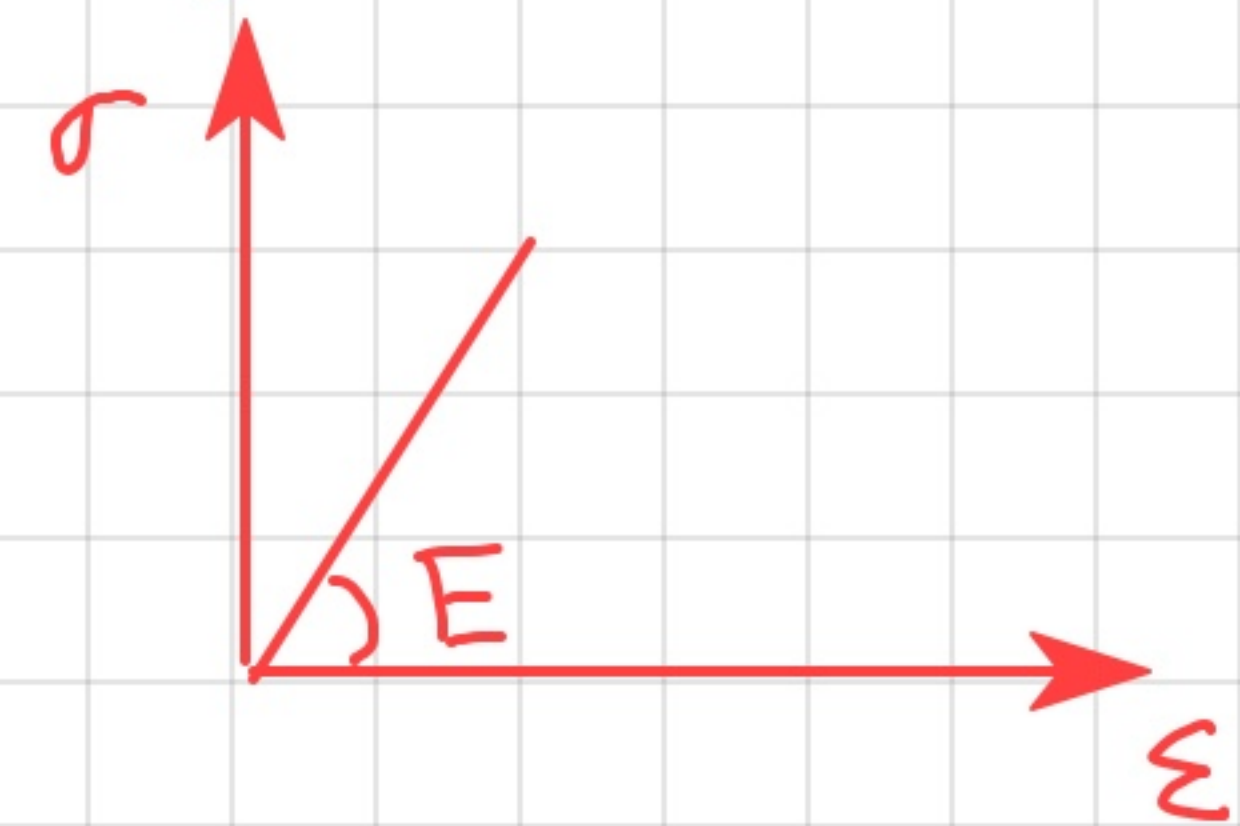
断面積 A



$$\frac{N}{A} = E \cdot \frac{\Delta L}{L}$$

$$\Delta L = \frac{N \cdot L}{A \cdot E}$$

$$\begin{aligned}\pi &= \text{弾性} \times \text{変形} \\ \sigma &= E \epsilon\end{aligned}$$



$$\sigma = \frac{N}{A}, \quad \epsilon = \frac{\Delta L}{L}$$

○
N_{ad}

$$\begin{aligned}\Sigma F (=0 \delta') \\ N_{ad} + P + P - 2P &= 0 \\ N_{ad} &= 0\end{aligned}$$

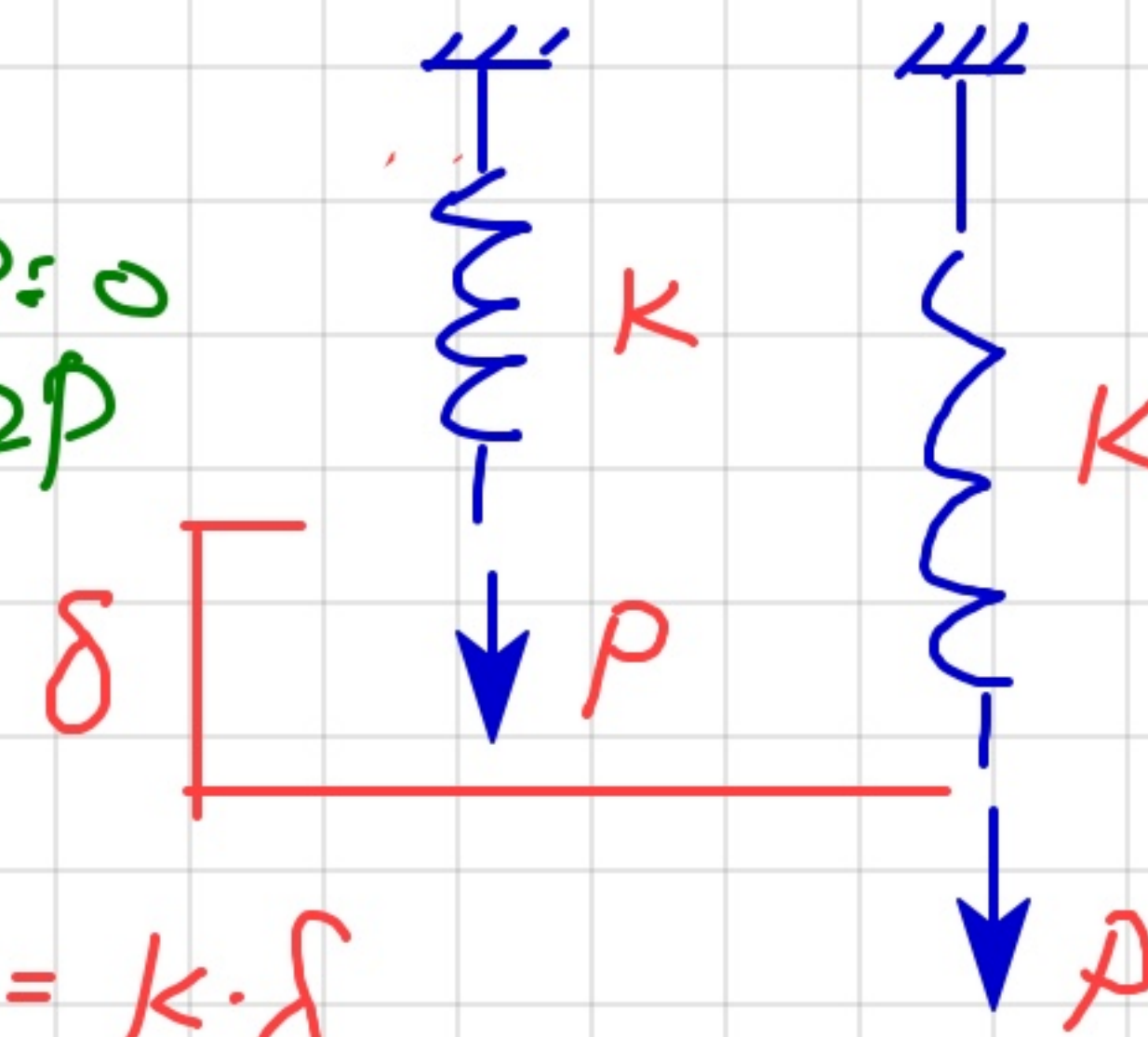
○
N_{ab}

$$\begin{aligned}\Sigma F (=0 \delta') \\ N_{ab} + P - 2P &= 0 \\ N_{ab} &= P\end{aligned}$$

2P

○
N_{bc}

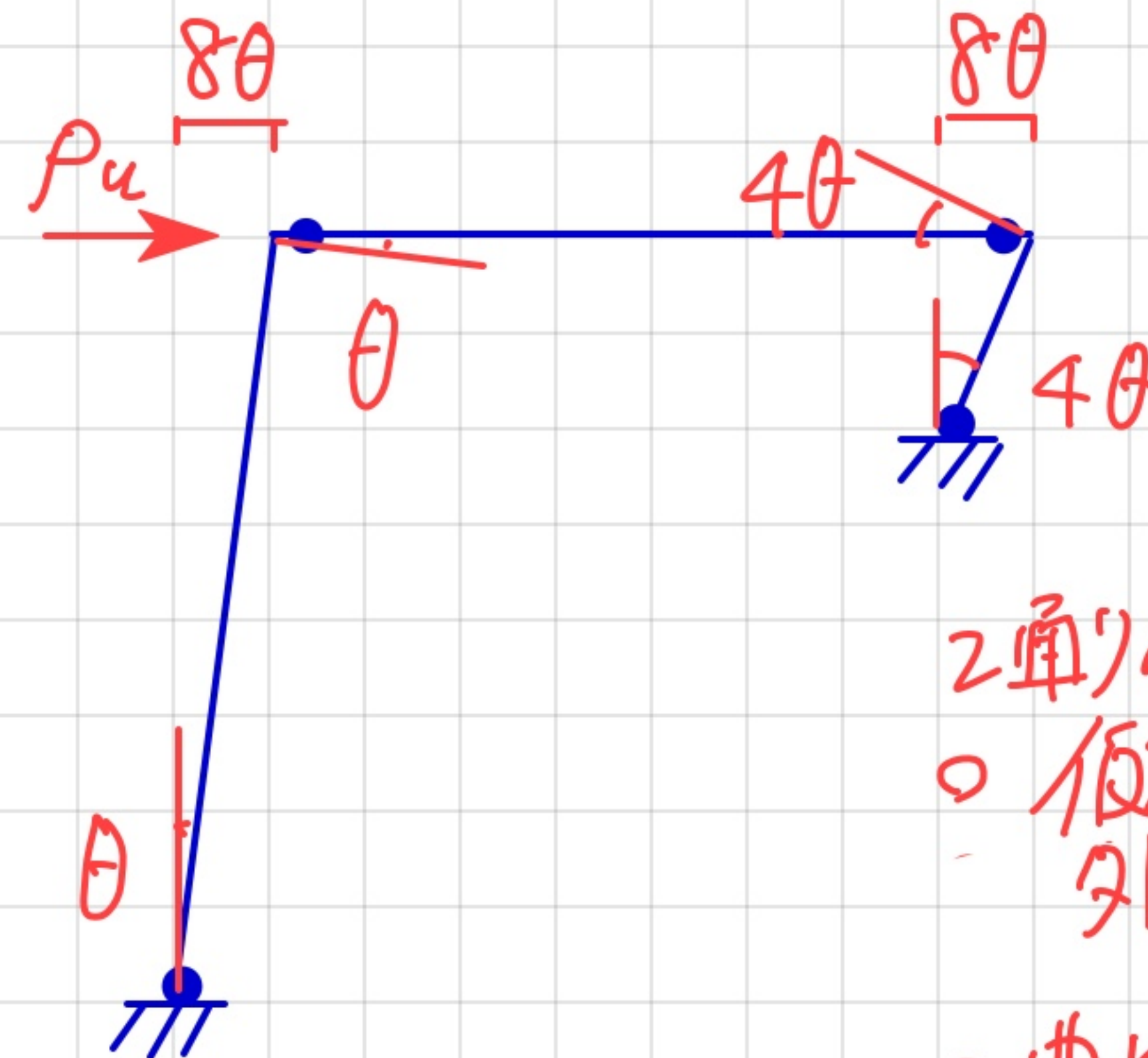
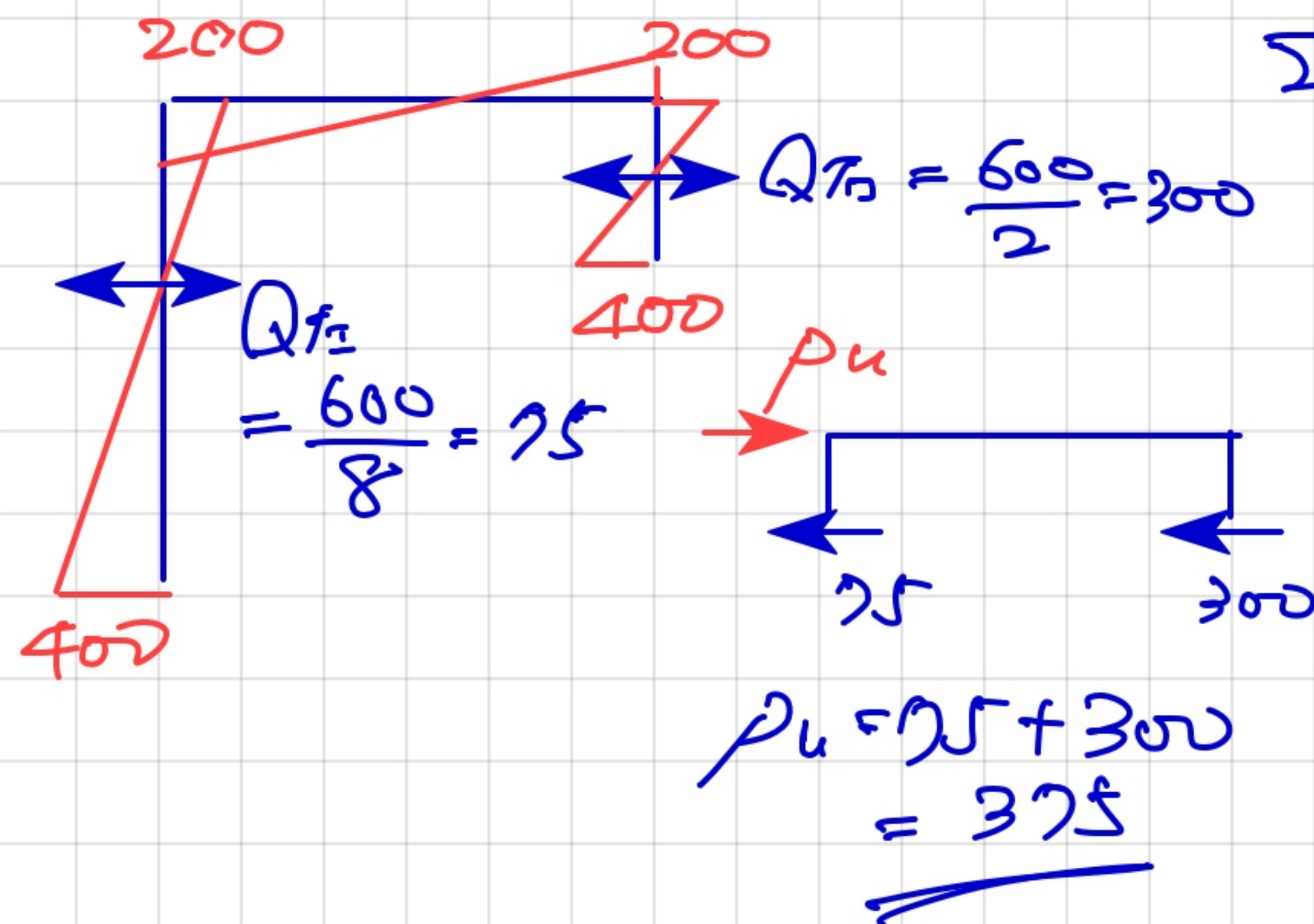
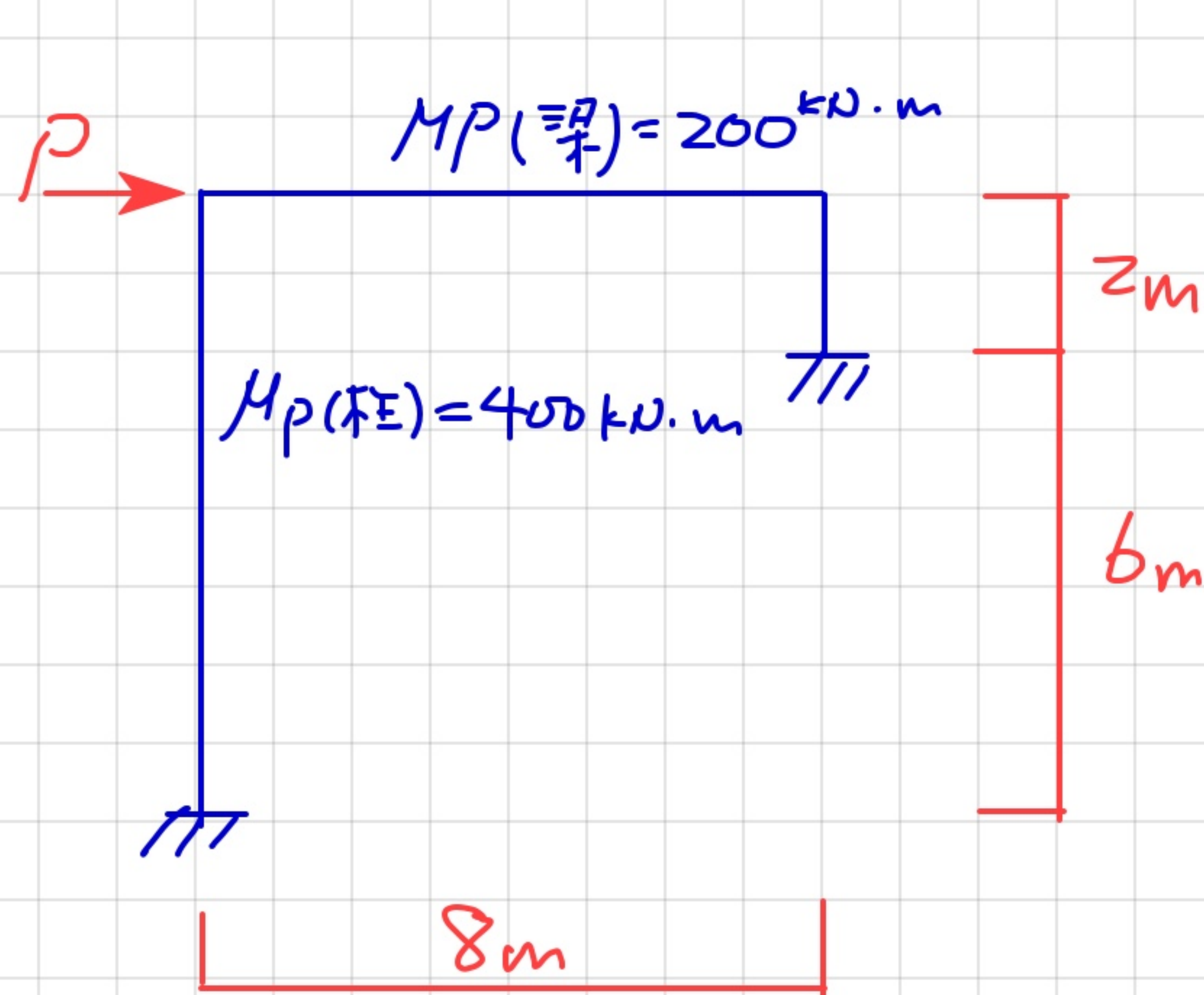
$$\begin{aligned}\Sigma F (=0 \delta') \\ N_{bc} - 2P &= 0 \\ N_{bc} &= 2P\end{aligned}$$



$$P = K \cdot \delta$$

$$\pi = \text{弾性} \times \text{変形}$$

No3 崩壊荷重を求める問題 (H27-4の類似)



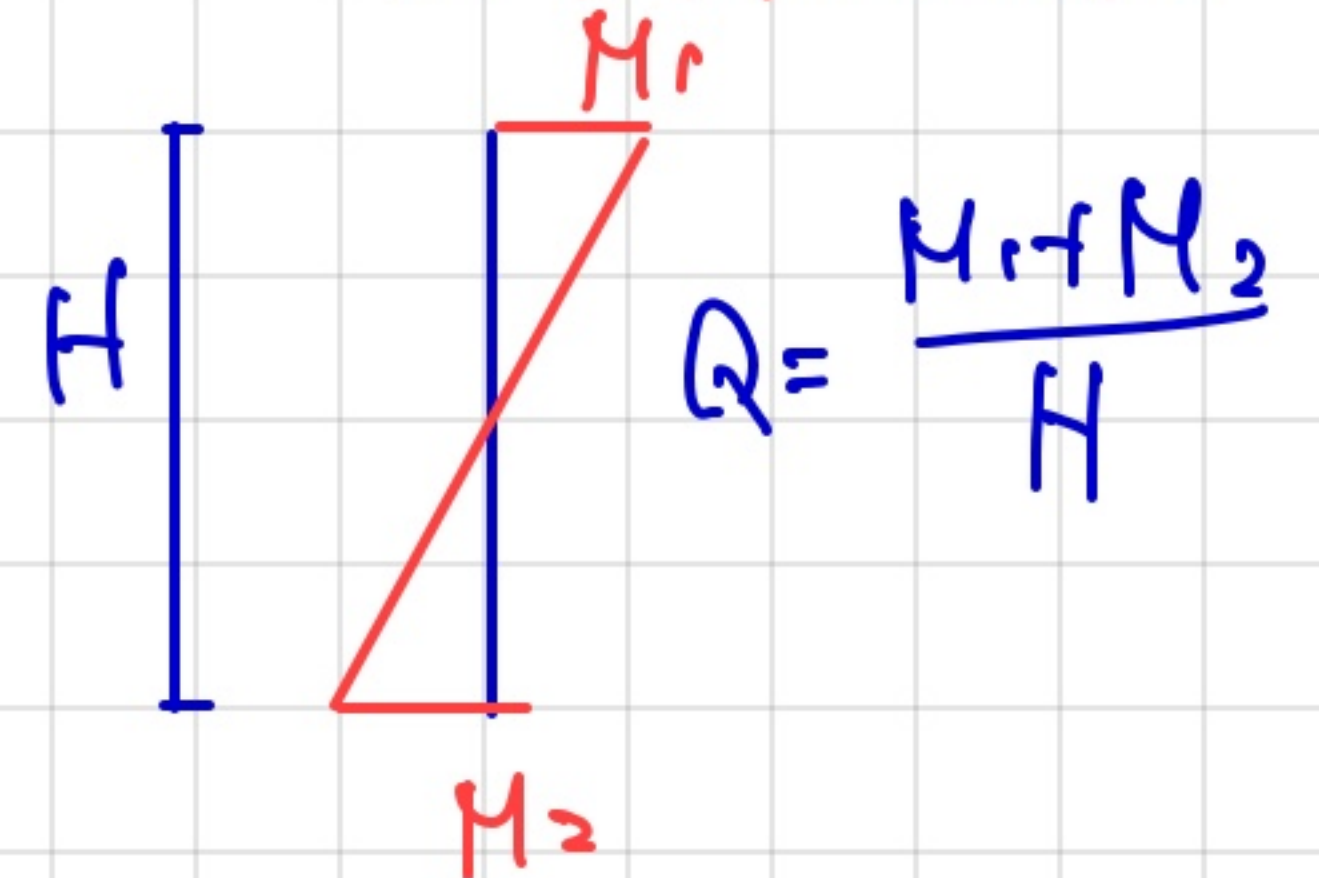
$$\Sigma P \delta = P_u 8 \theta$$

$$\Sigma M \theta = 400 \theta + 200 \theta + 200 \times 4 \theta + 400 \times 4 \theta = 3000 \theta$$

$$8 P_u \theta = 3000 \theta \quad \underline{\underline{P_u = 375}}$$

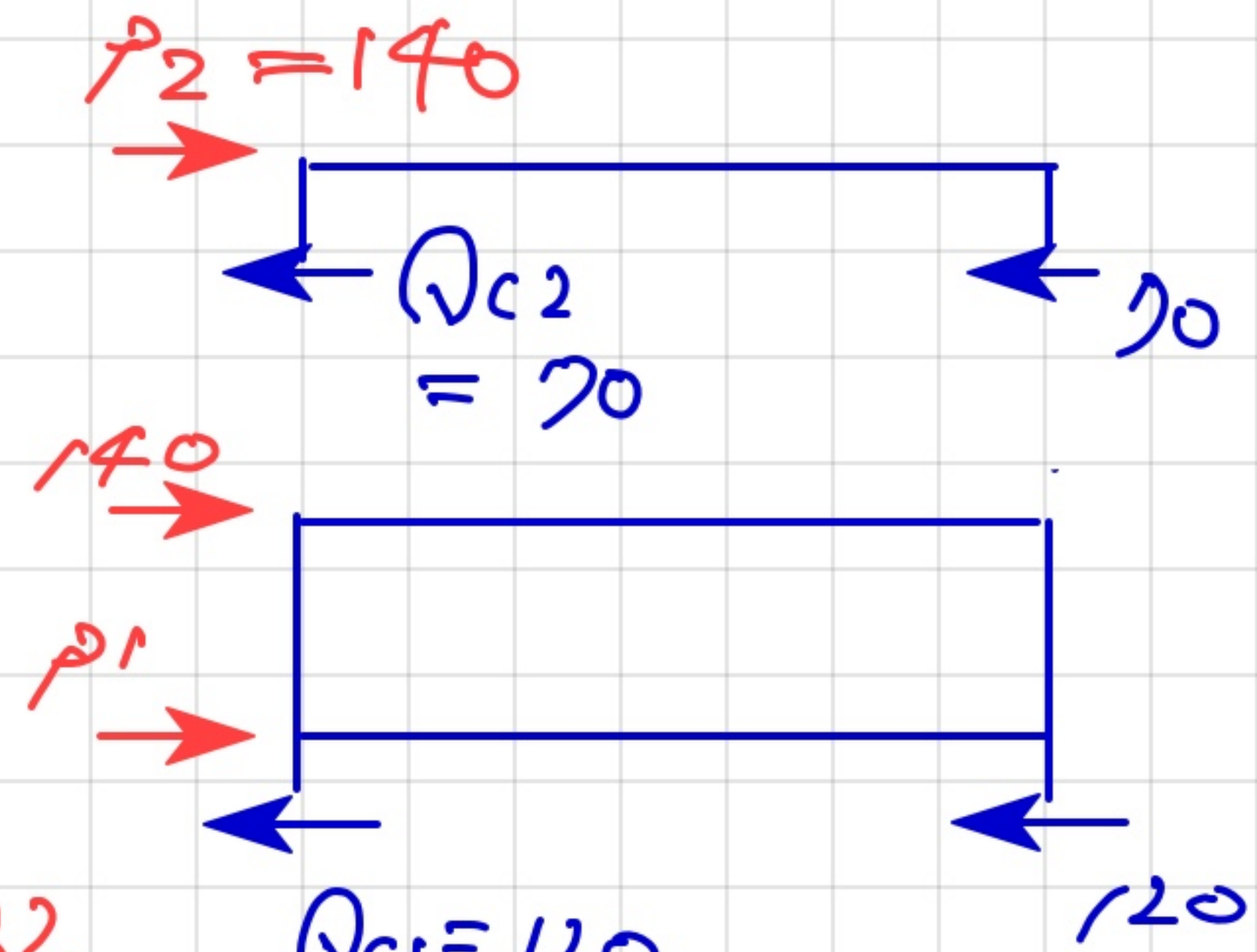
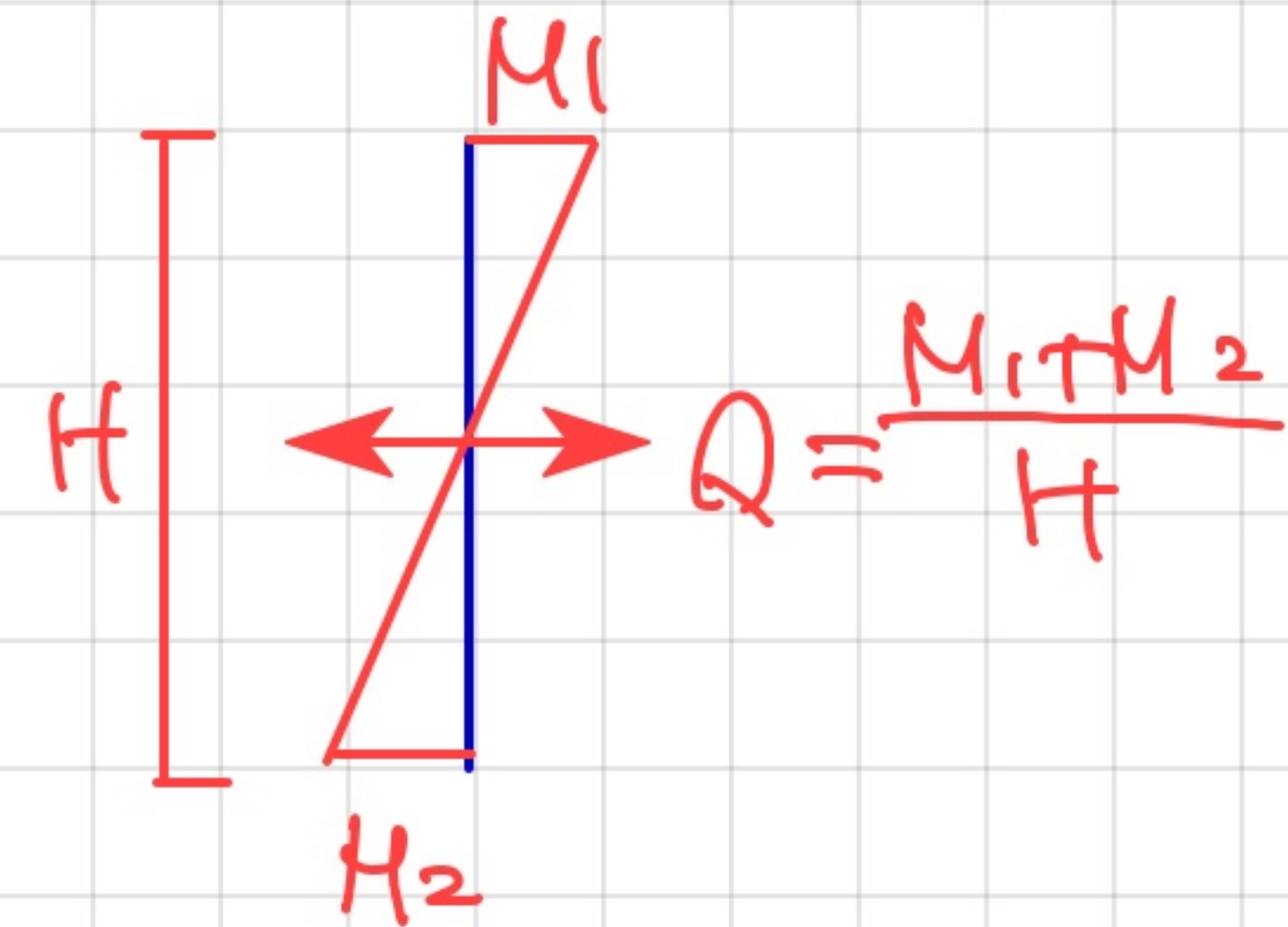
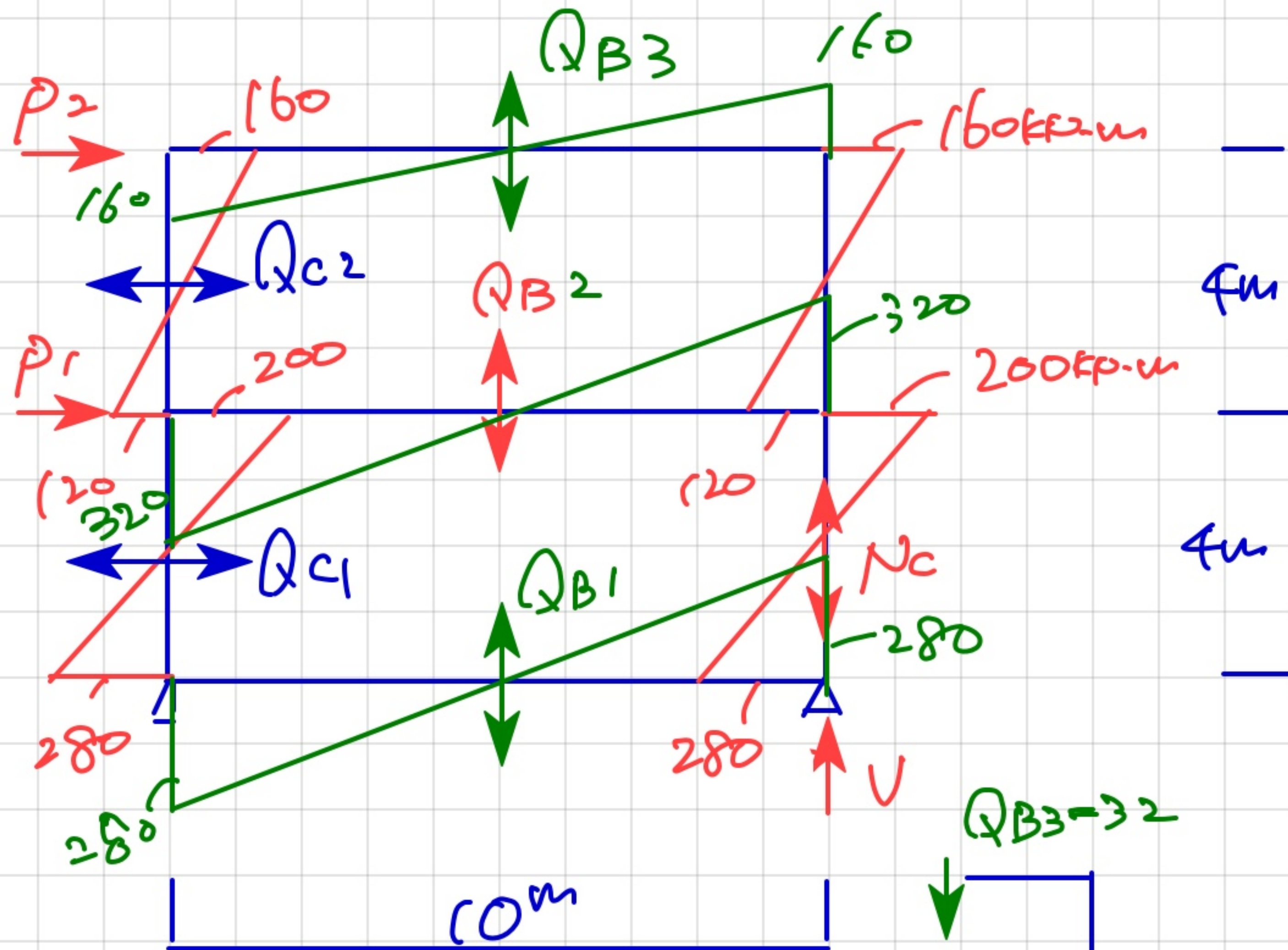
2通りの解き方
 ○ 仮想仕事の原理
 外力の仕事 = 内力の仕事

○ 曲げモーメントとせん断力の関係



№04. 桁梁ラ-γ=0の応力・反力を求める問題 (H30-4の類似)

曲げモーメントとせん断力の関係



$$Q_{C2} = 280 / 4 = 70$$

$$Q_{C1} = 480 / 4 = 120$$

$$Q_{B3} = 320 / 10 = 32 \quad (2)$$

$$Q_{B2} = 640 / 10 = 64$$

$$Q_{B1} = 560 / 10 = 56$$

$$Q_{B3} = 32$$

$$Q_{B2} = 64$$

$$Q_{B1} = 56$$

$$Q_{B2} = 64$$

$$N_C = 64 + 32 = 96 \quad (3)$$

$$V = 32 + 64 + 56 = 152 \quad (4)$$

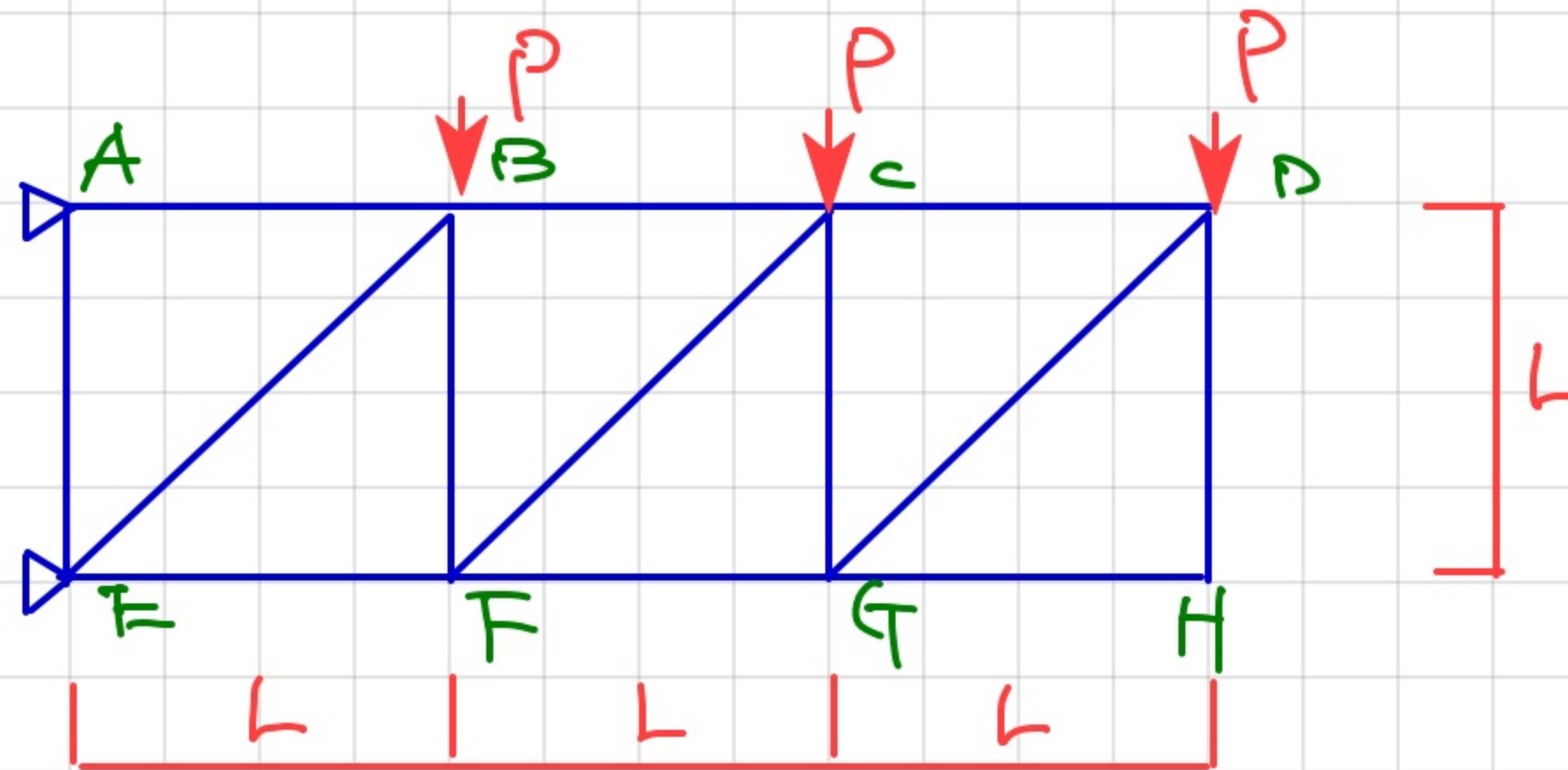
$$Q_{C1} = 120$$

$$\Sigma X = 0 \text{ (or)}$$

$$P_1 + 140 - 120 - 120 = 0$$

$$P_1 = 100 \quad (1)$$

105 152 の塑性崩壊荷重の問題 (H22-05 の類似)

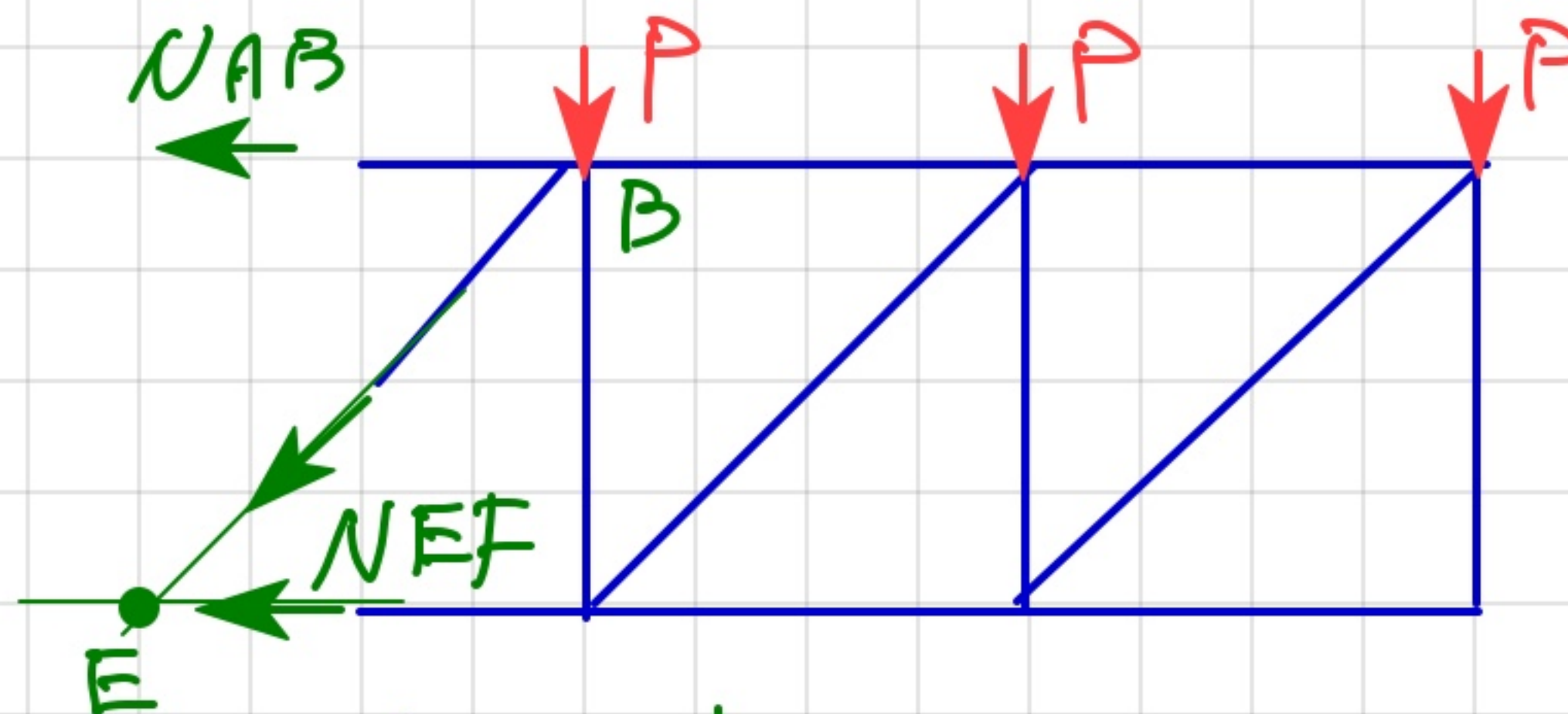


一つの部材が
降伏応力度 σ_y に達した時が
塑性崩壊

応力度 $= N/A$ (単位面積
あたりの力)

最大応力の箇所が最初に降伏する

↓
最大応力の箇所の N を求める



$$\sum M_E = 0 \text{ (ccw)}$$

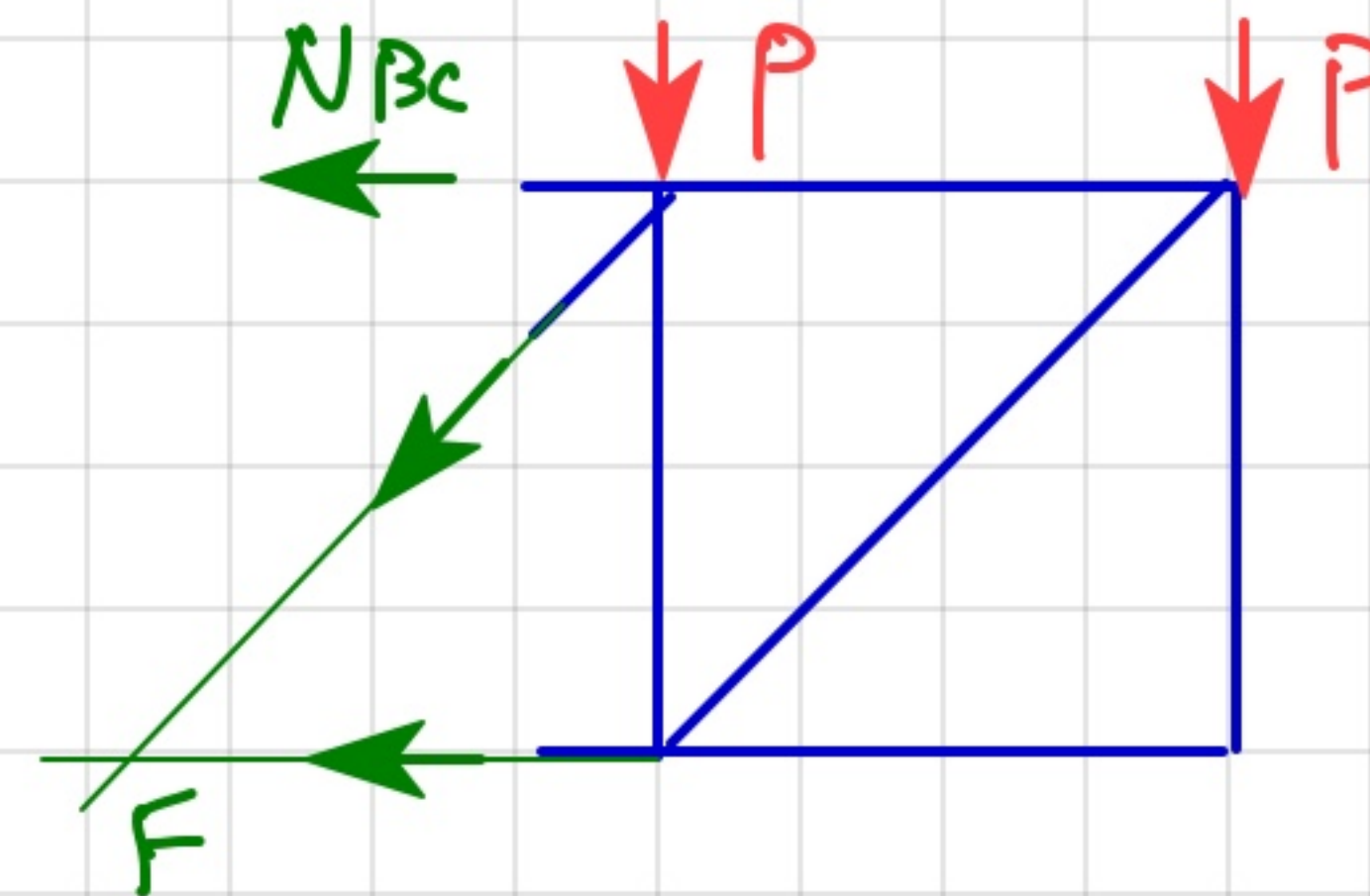
$$-N_{AB} \times L + P \times L + P \times 2L + P \times 3L = 0$$

$$N_{AB} = 6P$$

$$\sum M_B = 0 \text{ (ccw)}$$

$$N_{EF} \times L + P \times L + P \times 2L = 0$$

$$N_{EF} = -3P$$



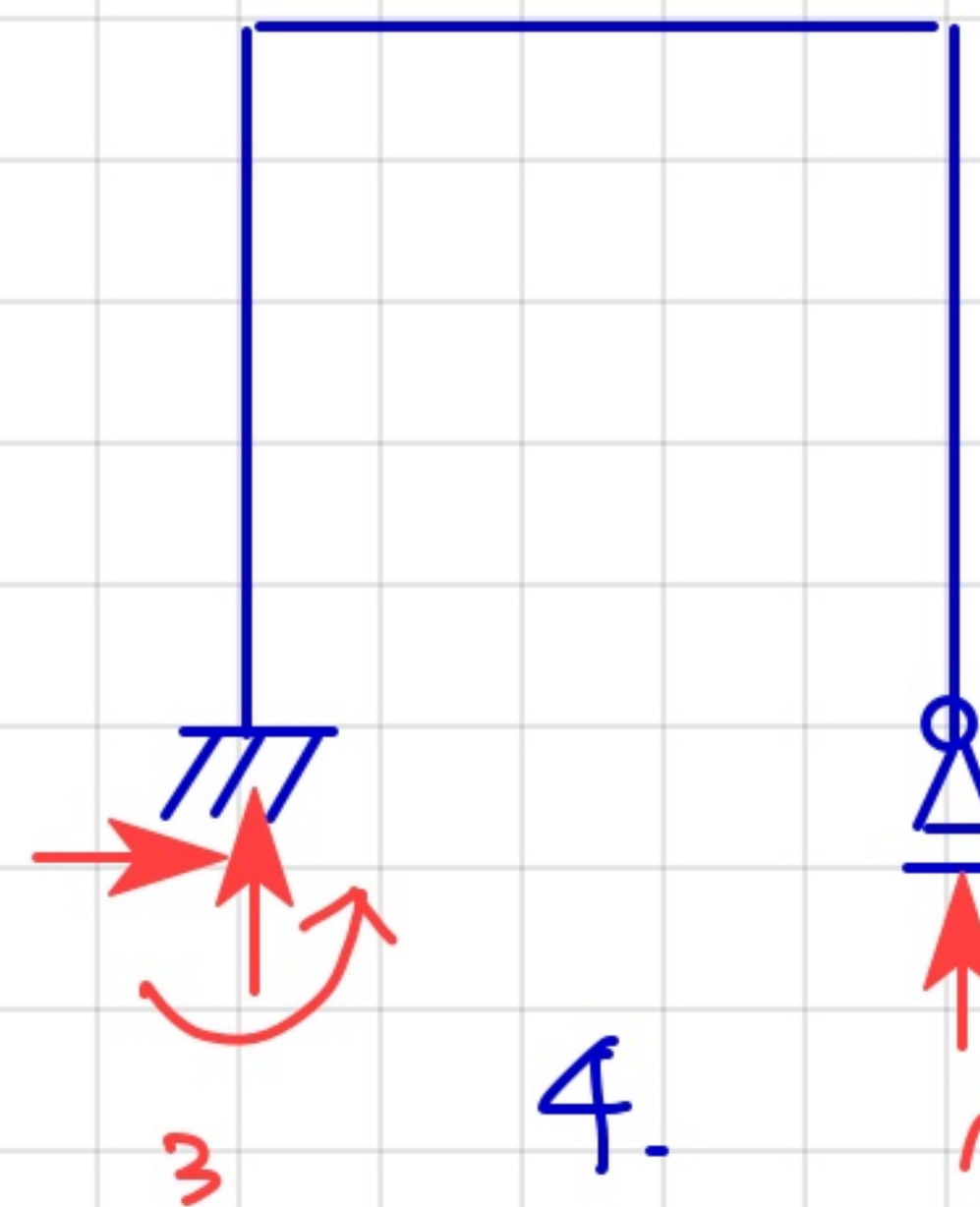
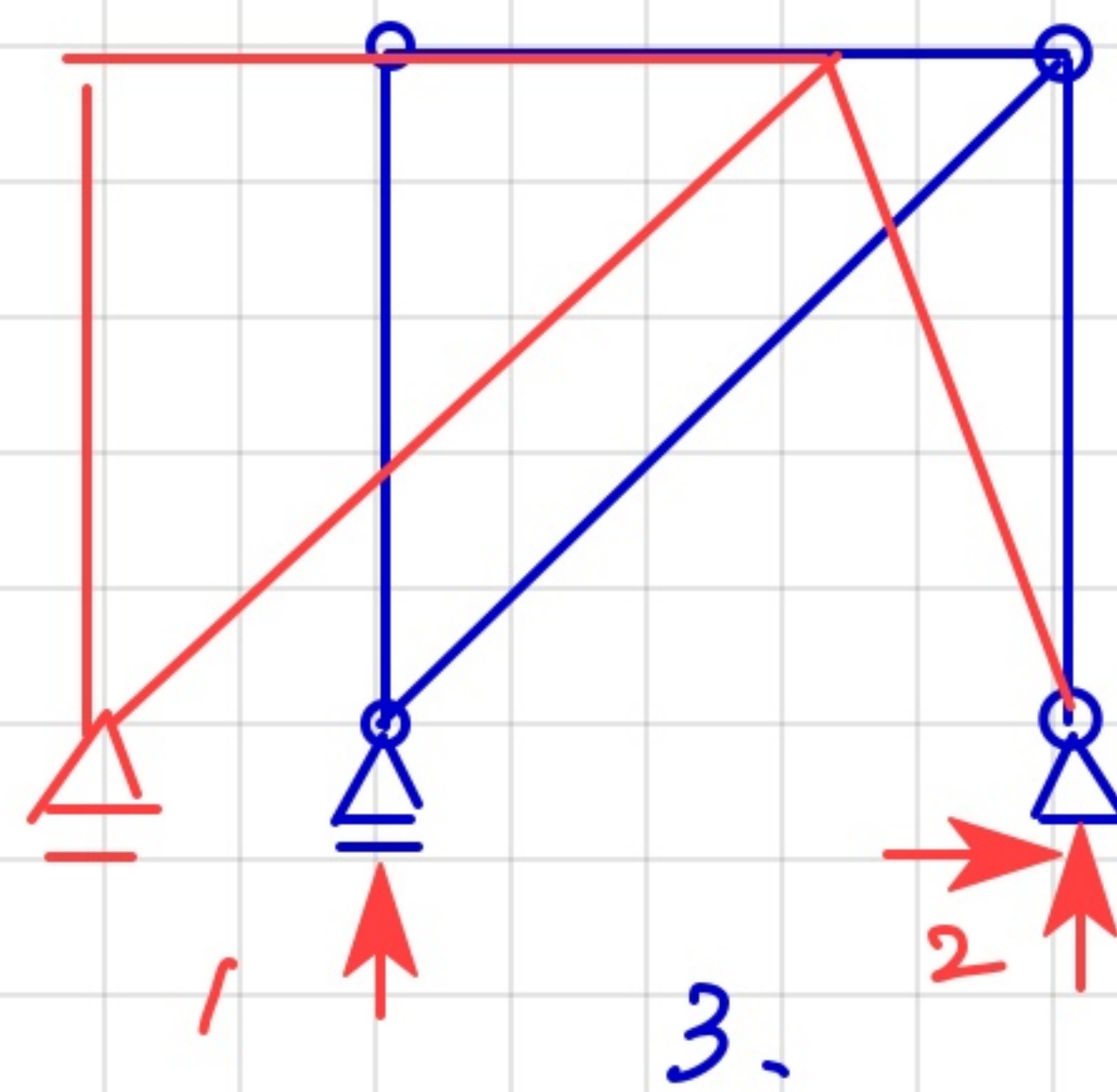
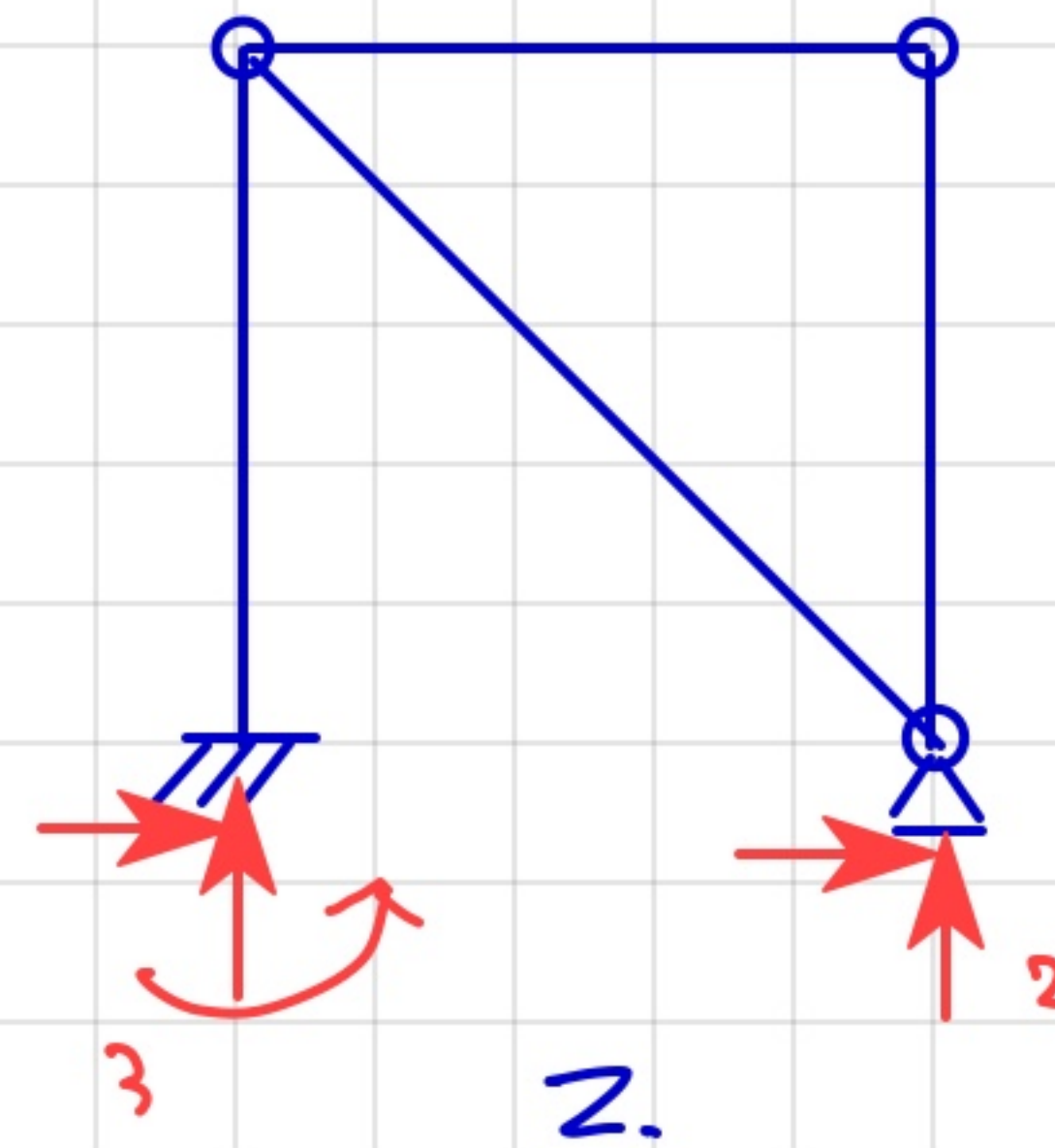
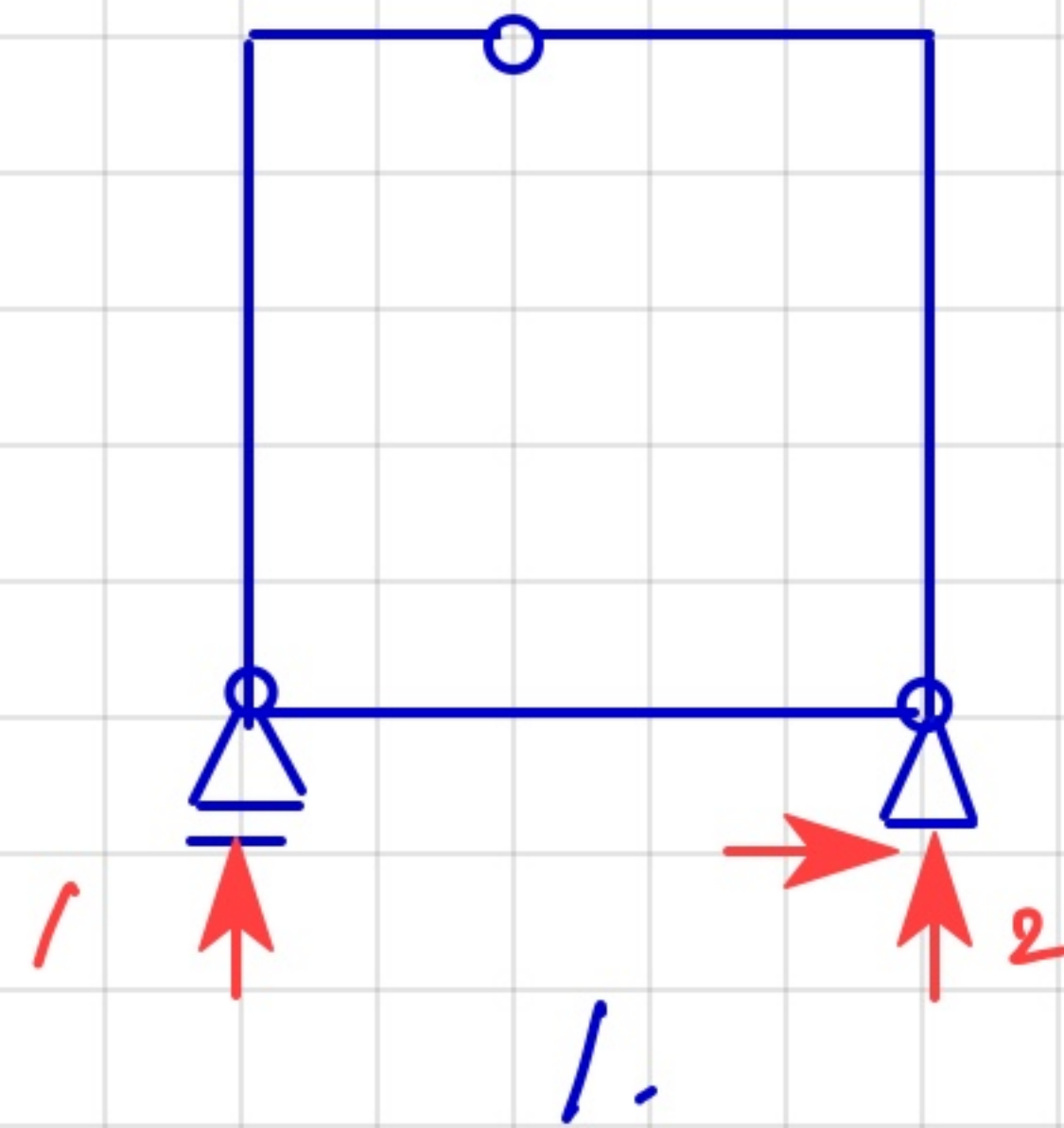
$$\sigma_y = \frac{N}{A} = \frac{6P}{A} \Rightarrow P = \frac{A \cdot \sigma_y}{6}$$

No6 静定構造と変形問題 (201-06の類似)

判別式 $n + s + r - 2k$

n : 反力数
 s : 部材数
 r : 剛節接部材数
 k : 節点数

判別式 0 : 静定
 > 0 : 不静定
 < 0 : 不安定



$n + s + r - 2k$

1	$3 + 5 + 2 - 10 = 0$
2	$5 + 4 + 0 - 8 = 1$
3	$3 + 4 + 0 - 8 = -1$
4	$4 + 3 + 2 - 8 = 1$